

# CE 415

# DESIGN OF STEEL STRUCTURES

## LECTURE 12

## MIDTERM REVIEW (CONT.)

SEMESTER: SUMMER 2020

COURSE TEACHER: SAURAV BARUA

CONTACT NO: +8801715334075

EMAIL: [saurav.ce@diu.edu.bd](mailto:saurav.ce@diu.edu.bd)

# OUTLINE

- MIDTERM SYLLABUS

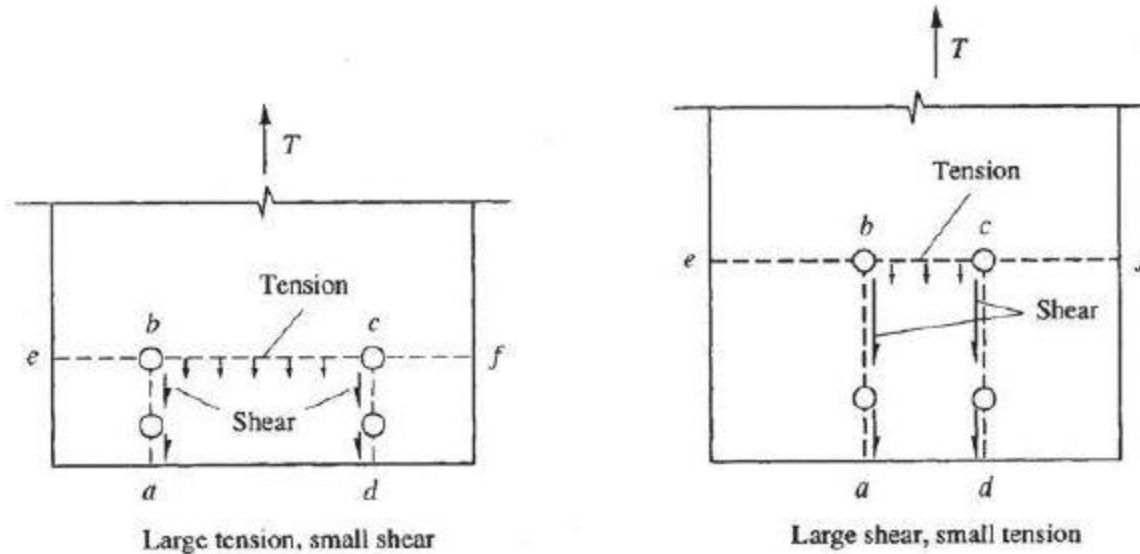
LECTURE 1 (16.5.20) – LECTURE 7 (13.6.20)

MARKS

25 MARKS

- 2 questions with a (Math), b and c (theory type) segments
- Time 2hr
- No alternatives options

## Block Shear Failure



**AISC-J4.3. defines two block shear failure modes:**

- 1. Rupture along the tensile plane (b-c in left fig.) accompanied by yielding along the shear planes (a-b and c-d in left fig.).**
- 2. Rupture along the shear planes (a-b and c-d right fig.) accompanied by rupture along the tensile plane (b-c in right fig.).**

## Block Shear Strength

The tensile failure is defined by *rupture along the net area* in both modes,

The failure along the shear planes can either be *Rupture along the net shear area* or *Yield along the gross shear area*, whichever is smaller.

Consistent with the limit states discussed earlier, the *gross area* is used for the yielding limit state and the *net area* is used for the fracture limit state.

Following the energy-of-distortion theory,

- Shear yield stress  $\tau_y$  is taken as  $0.6F_y$
- Shear strength/rupture  $\tau_u$  is taken as  $0.6F_u$

### The nominal block shear strength $T_n$ in tension

Shear yielding - tension rupture ( $\tau_y A_{gv} < \tau_u A_{nv}$ ) or ( $0.6F_y A_{gv} < 0.6F_u A_{nv}$ )

$$T_n = 0.6F_y A_{gv} + F_u U_{bs} A_{nt}$$

Shear fracture - tension rupture ( $\tau_y A_{gv} \geq \tau_u A_{nv}$ ) or ( $0.6F_y A_{gv} \geq 0.6F_u A_{nv}$ )

$$T_n = 0.6F_u A_{nv} + F_u U_{bs} A_{nt}$$

where

$A_{gv}$  : gross area acted upon by shear

$A_n$  : net area acted upon by tension

$A_{nv}$  : net area acted upon by shear

$F_u$  : specified (ASTM) minimum tensile strength

$F_y$  : specified (ASTM) minimum yield stress

When the tension stress is uniform, use  $U_{bs} = 1$ , where the tension is non-uniform use  $U_{bs} = 0.5$ .

## **RESIDUAL STRESS IN STEEL SECTIONS**

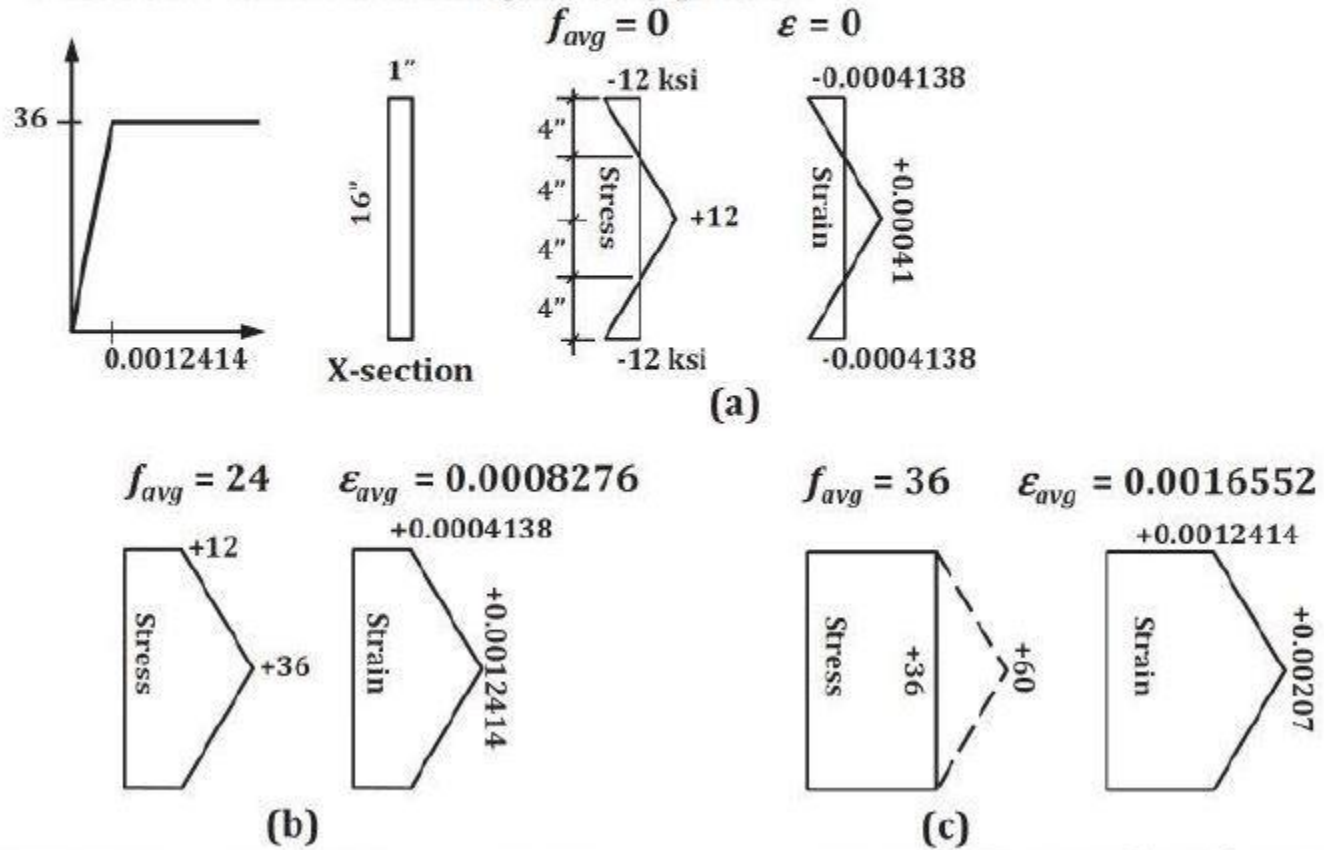
**Residual stresses are self balancing stresses that remain in a member without application of load after it has been formed/rolled into a finished product.**

### **Sources of residual stresses:**

- Uneven cooling which occurs after hot rolling of structural shapes**
- Cold bending or cambering during fabrication**
- Punching of holes and cutting operations during fabrication**
- Welding**

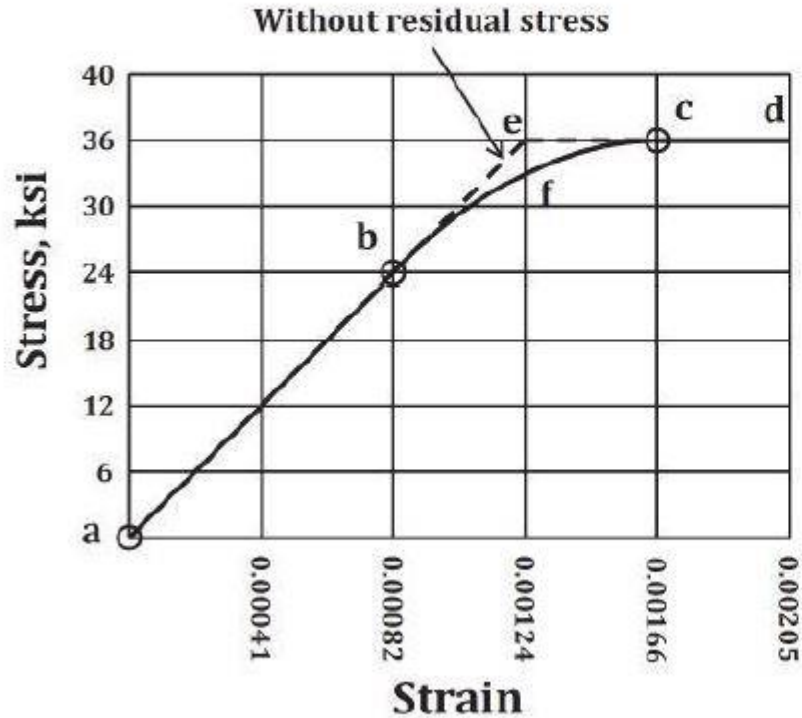
**Average tensile stress-strain relation of a 16"x1" x-section flat bar having residual stress.**

Distribution of residual stress and strain are shown in Fig. (A) when no external force acts. The bar is then gradually pulled.



## RESIDUAL STRESS IN STEEL SECTIONS

Average tensile stress-strain relation of a 16"x1" x-section flat bar having residual stress.



Due to presence of residual stress/strain the average stress-strain behavior follows path a-b-f-c-d

If there was no residual stress then the path would be a-b-e-c-d

@Saurav Barua

How calculate strain at 12 ksi stress?

copyright @Saurav Barua



## Equation of Stress-Strain Curve:

Up to point b, the stress-strain relation is linear. After c, the curve is flat. The transition from b to c can be covered by one parabolic curve as follows:

$f = k_1 \varepsilon^2 + k_2 \varepsilon + k_3$  Here,  $f$  is stress and  $\varepsilon$  is strain.

The constants  $k_1$ ,  $k_2$  and  $k_3$  can be found from three conditions:

- 1) At b,  $df/d\varepsilon = E = 29000$ , when  $\varepsilon = 0.0008276$  where  $E$  is the Young's modulus.
- 2) At b,  $f = 24$  when  $\varepsilon = 0.0008276$
- 3) At c,  $f = 36$  when  $\varepsilon = 0.0016552$

Now,  $df/d\varepsilon = 2k_1\varepsilon + k_2$ ,  $\therefore$  from (1),  $2k_1(0.0008276) + k_2 = 29000$  ----(1)

From (2),  $k_1(0.0008276)^2 + k_2(0.0008276) + k_3 = 24.0$  -----(2)

From (3),  $k_1(0.0016552)^2 + k_2(0.0016552) + k_3 = 36.0$  -----(3)

Solving the above three,

$$k_1 = -17520833.3, \quad k_2 = +58000.48, \quad k_3 = -12$$

Therefore,

$$f = +29000\varepsilon \quad \text{for } 0 \leq \varepsilon \leq 0.0008276 \text{ [portion a-b]}$$

$$f = -17520833.3\varepsilon^2 + 58000.48\varepsilon - 12 \quad \text{for } 0.0008276 \leq \varepsilon \leq 0.0016552 \text{ [portion b-f-c]}$$

$$f = +36 \quad \text{for } \varepsilon \geq 0.0016552$$

Check: ideally at c,  $df/d\varepsilon = 0$

$$\text{Check at e, } df/d\varepsilon = 2(-17520833.3)(0.0016552) + 58000.48$$

$$= -0.49 \rightarrow \text{very small compared to } E = 29000 \rightarrow \text{OK.}$$