

CE 415

DESIGN OF STEEL STRUCTURES

LECTURE 10

WELDING (CONT.)

SEMESTER: SUMMER 2021

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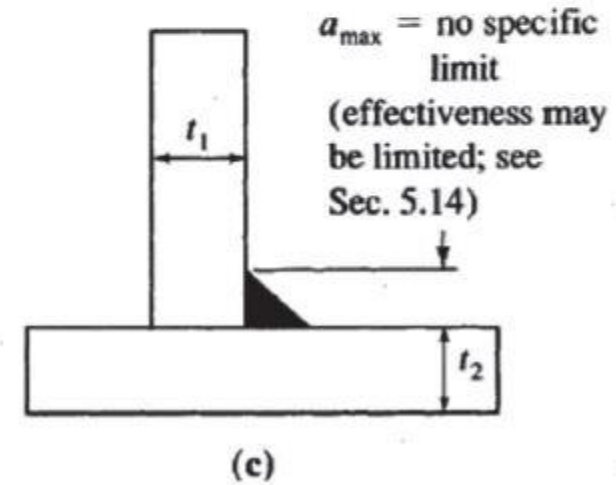
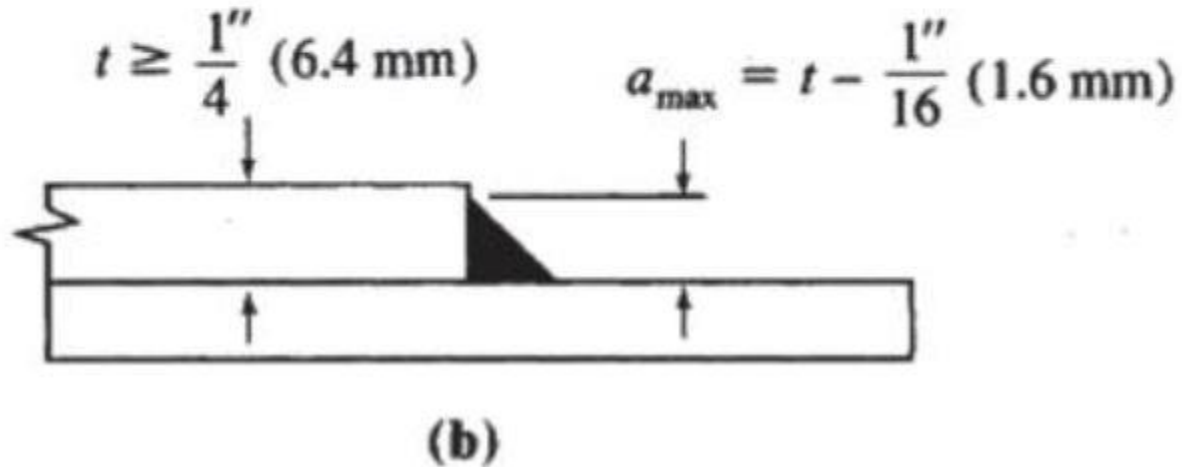
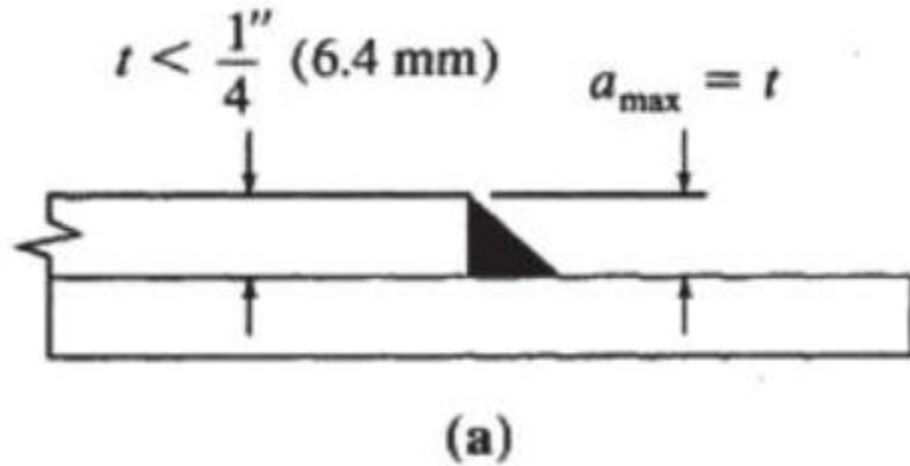
OUTLINE

- Defects of welding
- Fillet weld
- Strength of weld (LRFD)
- Strength of weld (ASD)
- Design capacity of weld member

Possible defects of Welds

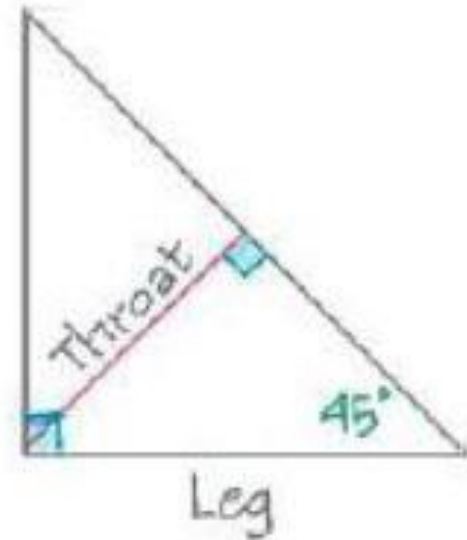
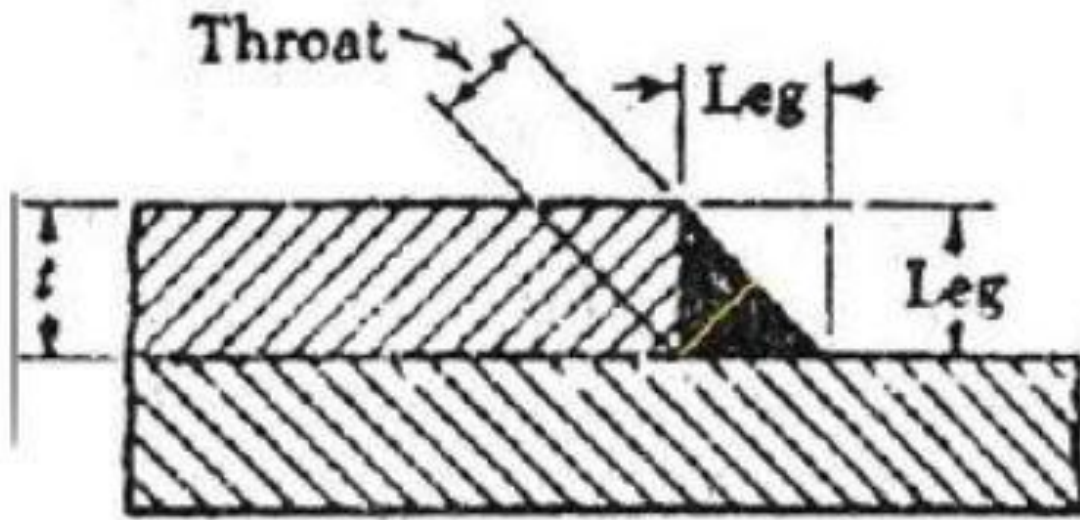
Unless good welding techniques and procedures are used, a number of possible defects may result relating to discontinuities within the weld. Some of the common defects are:

- Incomplete fusion**
- Inadequate joint penetration**
- Porosity**
- Undercutting**
- Inclusion of slag**
- Cracks.**



- (a) If plate thickness $t < 1/4''$, then maximum weld thickness, $a(\max) = t$
- (b) If plate thickness $t \geq 1/4''$, then $a(\max) = t - 1/16''$ and $a(\min) = 1/4''$

Fillet Weld



Effective throat size, $t_e = 0.707a$, where a = size of weld.

NOMINAL STRENGTH OF WELDS

Fillet Welds: The nominal strength *per unit length* of weld, R_{nw} , is expressed as,

a. For the weld metal,

$$R_n = F_w A_w = 0.60 F_{EXX} t_e (1 \text{ in.}) = 0.60 t_e F_{EXX} \quad (5.13.1)$$

b. For the base metal,

$$R_n = F_{BM} A_{BM} = 0.6 F_y t (1 \text{ in.}) = 0.6 t F_y \quad \text{yielding} \quad (5.13.2)$$

$$R_n = F_{BM} A_{BM} = 0.6 F_u t (1 \text{ in.}) = 0.6 t F_u \quad \text{rupture} \quad (5.13.3)$$

FILLET WELD STRENGTH: LRFD

The design strength (AISC-J2.4.) *per unit length* of a fillet weld is based on the shear resistance through the throat of the weld, regardless of the direction of the applied load, as follows:

$$\phi R_{nw} = 0.75t_e(0.60 F_{EXX}) \quad (5.14.4)$$

but not greater than the shear yield or shear rupture strengths of the adjacent base materials. The base material design strength is the lower value of:

$$\phi R_n = 1.0(0.6F_y)A_{BM} = 0.6tF_y \quad \text{yield strength} \quad (5.14.5)$$

$$\phi R_n = 0.75(0.6F_u)A_{BM} = 0.45tF_u \quad \text{rupture strength} \quad (5.14.6)$$

t_e = effective throat dimension (see Sec. 5.12)

F_{EXX} = tensile strength of electrode material

t = thickness of base material along which weld is placed

F_u = tensile strength of base metal

FILLET WELD STRENGTH: ASD

The design strength (AISC-J2.4.) *per unit length* of a fillet weld is based on the shear resistance through the throat of the weld, regardless of the direction of the applied load, as follows:

$$\frac{R_{nw}}{\Omega} = \frac{F_w A_w}{\Omega} = \frac{(0.6 F_{EXX})(t_e \times 1)}{\Omega} = \frac{0.6 F_{EXX} t_e}{2.00}$$

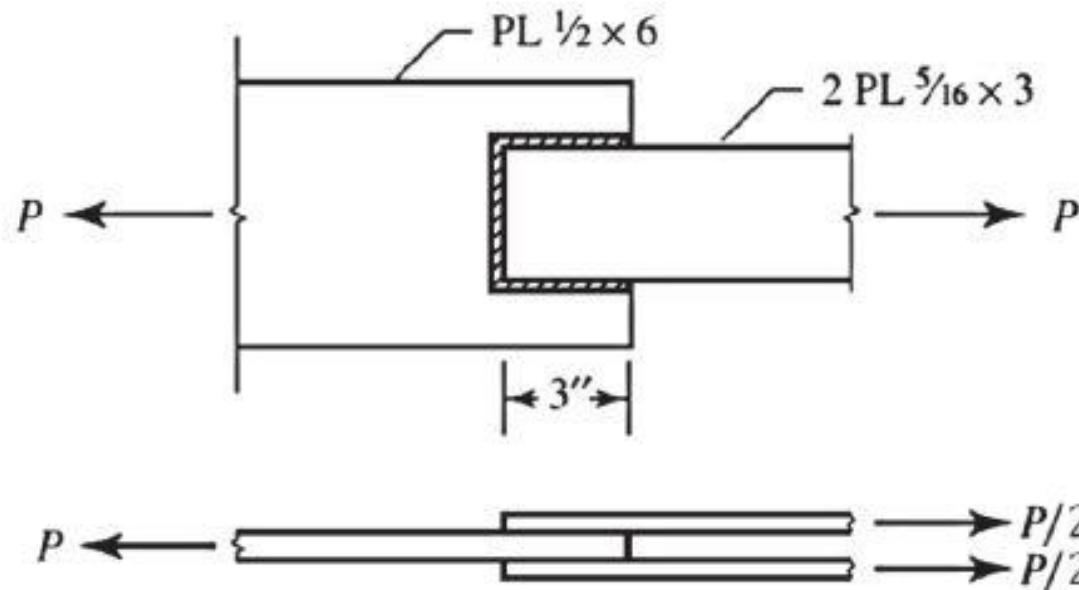
but not greater than the shear yield or shear rupture strengths of the adjacent base materials. The base material design strength is the lower value of:

$$\frac{R_n}{\Omega} = \frac{F_y A_{BM}}{\Omega} = \frac{(0.6 F_y)(t \times 1)}{\Omega} = \frac{0.6 t F_y}{1.50} \quad \text{yield strength}$$

$$\frac{R_n}{\Omega} = \frac{F_{vu} A_{BM}}{\Omega} = \frac{(0.6 F_u)(t \times 1)}{\Omega} = \frac{0.6 t F_u}{2.00} \quad \text{rupture strength}$$

Example

A tension member splice is made with $\frac{1}{4}$ -inch E70 fillet welds as shown in Figure. Each side of the splice is welded as shown. The inner member is a PL $\frac{1}{2} \times 6$ and each outer member is a PL $\frac{5}{16} \times 3$. All steel is A36. Determine the maximum design capacity, ϕP_n , based on weld limit states.



All plates A36:

$$F_y = 36 \text{ ksi}, F_u = 58 \text{ ksi.}$$

Weld strength: $F_{EXX} = 70 \text{ ksi.}$

$$\begin{aligned} \text{Weld length on each side} &= 3 + 3 + 3 \\ &= 9 \text{ inch.} \end{aligned}$$

Weld size, $s = \frac{1}{4} \text{ inch}$

$$\therefore \text{Throat } t_e = s / \sqrt{2} = 0.25 / 1.414$$

$$\therefore = 0.177 \text{ inch}$$

Fillet weld capacity: $\phi P_n = \phi R_{nw}L = 0.75[t_e(0.6F_{EXX})]L$
 $= 0.75 \times 0.177 \times 0.6 \times 70 \times (9+9) = 100.4 \text{ kip}$

Base metal - Inner Plate:

Yielding: $\phi P_n = \phi R_nL = 1.0(0.6tF_y)L = 0.6 \times 1/2 \times 36 \times 9 = 97.2 \text{ kip.}$

Rupture: $\phi P_n = \phi R_nL = 0.75(0.6tF_u)L = 0.75 \times 1/2 \times 58 \times 9 = 117.45 \text{ kip.}$

Base metal - Outer Plate:

Yielding: $\phi P_n = \phi R_nL = 1.0(0.6tF_y)L = 0.6 \times 5/16 \times 36 \times 9 \times 2 = 121.5 \text{ kip.}$

Rupture: $\phi P_n = \phi R_nL = 0.75(0.6tF_u)L = 0.75 \times 5/16 \times 58 \times 9 \times 2 = 117.45 \text{ kip.}$

\therefore Base metal (Inner plate) yielding governs.

$\therefore \phi P_n = 97.2 \text{ kip. Ans.}$