

# CE 415

# DESIGN OF STEEL STRUCTURES

## LECTURE 3

## TENSION MEMBER

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## LRFD Design

$$\phi T_n \geq T_u$$

Where,

$\phi_t$  = resistance factor relating to tension member strength

$T_n$  = nominal strength of a tension member (see AISC-Chapter D)

$\phi_t T_n$  = design strength of a tension member

$T_u$  = factored load on a tension member

### Tension Members have THREE limit states:

1. Yielding on gross section
2. Fracture on effective section
3. *Block Shear*

#### Yielding on gross section

$$\phi T_n = \phi_t F_y A_g = 0.9 F_y A_g$$

#### Fracture on effective net section

$$\phi T_n = \phi_t F_u A_e = 0.75 F_u A_e$$

Note that the resistance factor  $\phi_t$  is 0.90 for the yielding limit state and 0.75 for the fracture limit state.

## ASD Design

### Yielding on gross section

Nominal Strength:  $T_n = F_y A_g$

Allowable Strength:  $\frac{T_n}{\Omega} = \frac{F_y A_g}{\Omega} = \frac{F_y A_g}{1.67}$

Safety factor  $\Omega = 1.67$  for yielding on gross section

### Fracture on effective net section

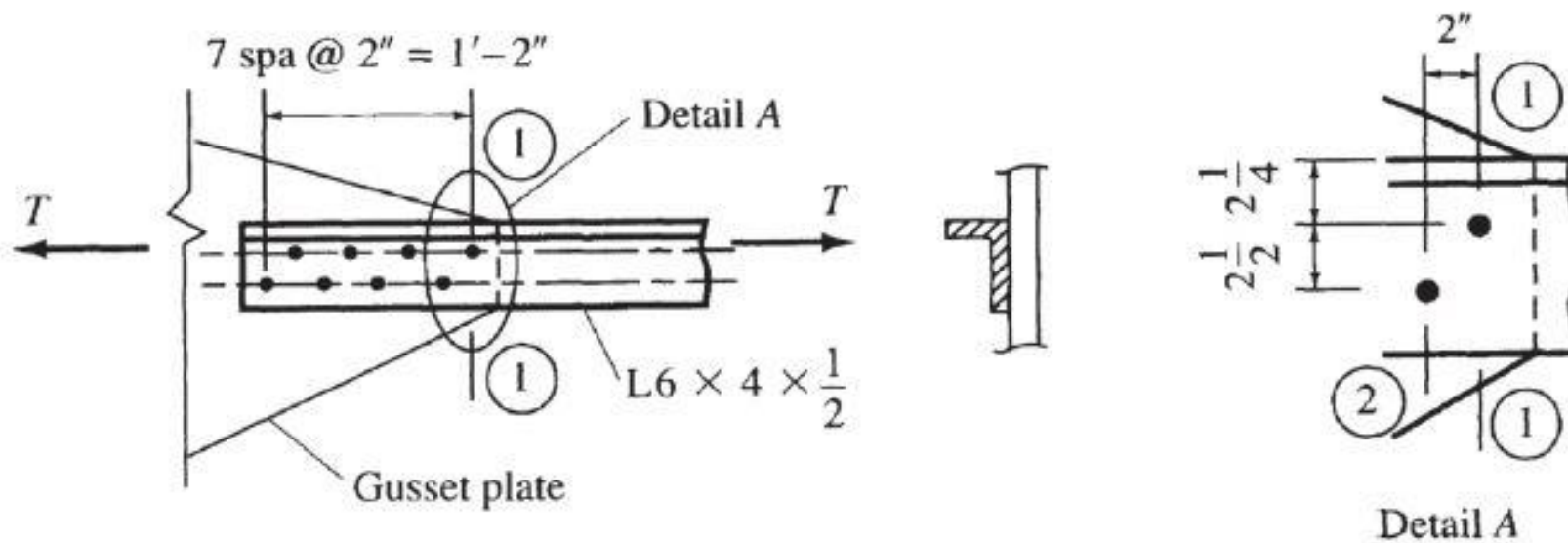
Nominal Strength:  $T_n = F_u A_e$

Allowable Strength:  $\frac{T_n}{\Omega} = \frac{F_u A_e}{\Omega} = \frac{F_u A_e}{2.00}$

Safety factor  $\Omega = 2.00$  for fracture on net section

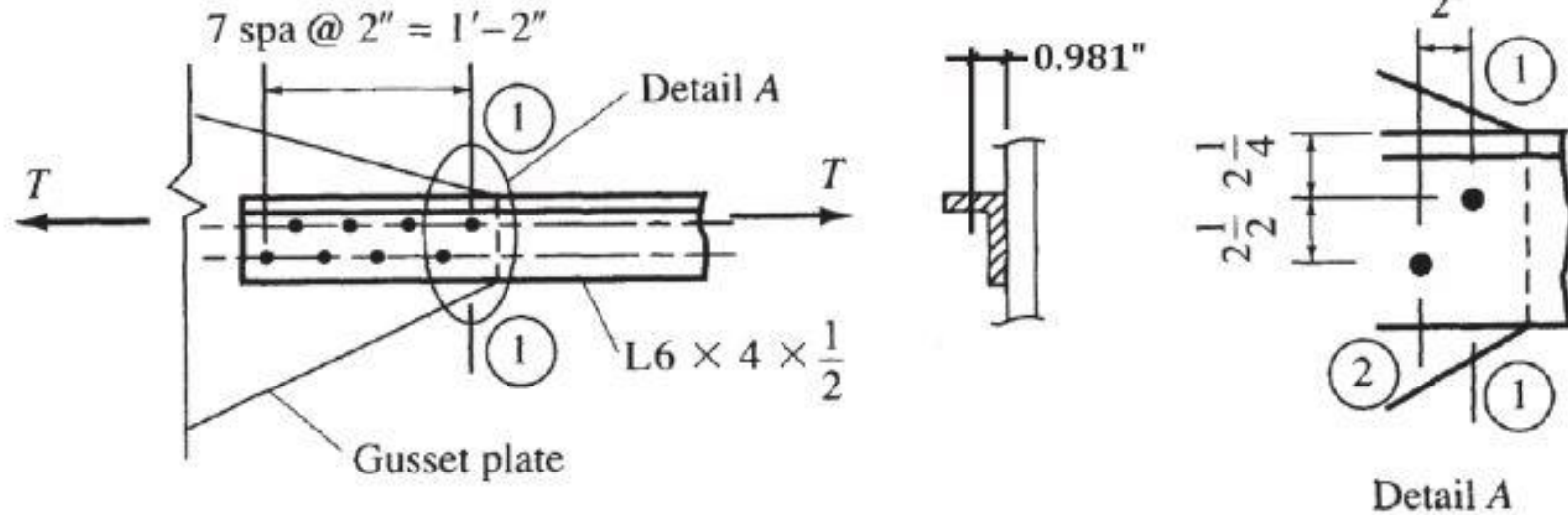
## ASD Load Capacity

Determine the service load capacity in tension for an  $L6 \times 4 \times \frac{1}{2}$  A572 Grade 50 steel connected with  $\frac{7}{8}$ -in.-diam. bolts in standard holes as shown. Assume live load to be three times the dead load. Neglect block shear failure.



Designation				A	$I_x$	$r_x$	y	$I_y$	$r_y$	x
L 9x4x1	9 x	4 x	1	12.0	97.0	2.84	3.50	12.0	1.00	1.00
L 7x4x0.625	7 x	4 x	5/8	6.48	32.4	2.24	2.46	7.84	1.10	0.963
L 6x4x0.75	6 x	4 x	3/4	6.94	24.5	1.88	2.08	8.68	1.12	1.08
L 6x4x0.625	6 x	4 x	5/8	5.86	21.1	1.90	2.03	7.52	1.13	1.03
L 6x4x0.5625	6 x	4 x	9/16	5.31	19.3	1.90	2.01	6.91	1.14	1.01
L 6x4x0.5	6 x	4 x	1/2	4.75	17.4	1.91	1.99	6.27	1.15	0.987
L 6x4x0.4375	6 x	4 x	7/16	4.18	15.5	1.92	1.96	5.60	1.16	0.964
L 6x4x0.375	6 x	4 x	3/8	3.61	13.5	1.93	1.94	4.90	1.17	0.941
L 4x3.5x0.437	4 x	3 1/2 x	7/16	3.09	4.76	1.24	1.23	3.40	1.05	0.978
L 4x3.5x0.375	4 x	3 1/2 x	3/8	2.67	4.18	1.25	1.21	2.95	1.06	0.955
L 4x3.5x0.312	4 x	3 1/2 x	5/16	2.25	3.56	1.26	1.18	2.55	1.07	0.932
L 4x3.5x0.25	4 x	3 1/2 x	1/4	1.81	2.91	1.27	1.16	2.09	1.07	0.909
L 4x3x0.625	4 x	3 x	5/8	3.98	6.03	1.23	1.37	2.87	0.849	0.871
L 4x3x0.5	4 x	3 x	1/2	3.25	5.05	1.25	1.33	2.42	0.864	0.827
L 4x3x0.4375	4 x	3 x	7/16	2.87	4.52	1.25	1.30	2.18	0.871	0.804
L 4x3x0.375	4 x	3 x	3/8	2.48	3.96	1.26	1.28	1.92	0.879	0.782
L 3.5x2.5x0.25	3 1/2 x	2 1/2 x	1/4	1.44	1.8	1.12	1.11	0.777	0.735	0.614
L 3x2.5x0.5	3 x	2 1/2 x	1/2	2.50	2.08	0.913	1.00	1.30	0.722	0.750
L 3x2.5x0.437	3 x	2 1/2 x	7/16	2.21	1.88	0.920	0.978	1.18	0.729	0.728
L 3x2.5x0.375	3 x	2 1/2 x	3/8	1.92	1.66	0.928	0.956	1.04	0.736	0.706
L 3x2.5x0.312	3 x	2 1/2 x	5/16	1.62	1.42	0.937	0.933	0.898	0.744	0.683
L 3x2.5x0.25	3 x	2 1/2 x	1/4	1.31	1.17	0.945	0.911	0.743	0.753	0.661

## ASD Load Capacity



Consider net section along 1-1 and along 1-2, because at these two sections, full force is transferred.

From AISC Manual:  $A_g = 4.75 \text{ in}^2$  and  $\bar{x} = 0.981$

Along 1-1:  $A_n = A_g - (1\text{-hole}) = 4.75 - (7/8 + 1/8) \times (1/2) = 4.25 \text{ in}^2$

Along 1-2:  $A_n = A_g - (2\text{-hole}) + s^2/4g$   
 $= 4.75 - 2(7/8 + 1/8) \times (1/2) + 2^2 / (4 \times 2.5) (1/2)$   
 $= 3.95 \text{ in}^2$

$U = 1 - \bar{x}/L = 1 - 0.981/14 = 0.93$

$A_e = UA_n = 0.93 \times 3.95 = 3.67 \text{ in}^2.$

## ASD Load Capacity

A572 Grade 50 steel:  $F_y = 50$  ksi,  $F_u = 65$  ksi.

### Yielding on gross area:

$$\text{Allowable tension} = T_n/\Omega = F_y A_g / \Omega = 50(4.75)/1.67 = 142 \text{ kips}$$

### Fracture on effective area:

$$\text{Allowable tension} = T_n/\Omega = F_u A_e / \Omega = 65(3.67)/2.00 = 119 \text{ kips} \rightarrow \text{Governs}$$

$$\text{Now, } D + L = 119$$

$$\Rightarrow D + 3D = 119$$

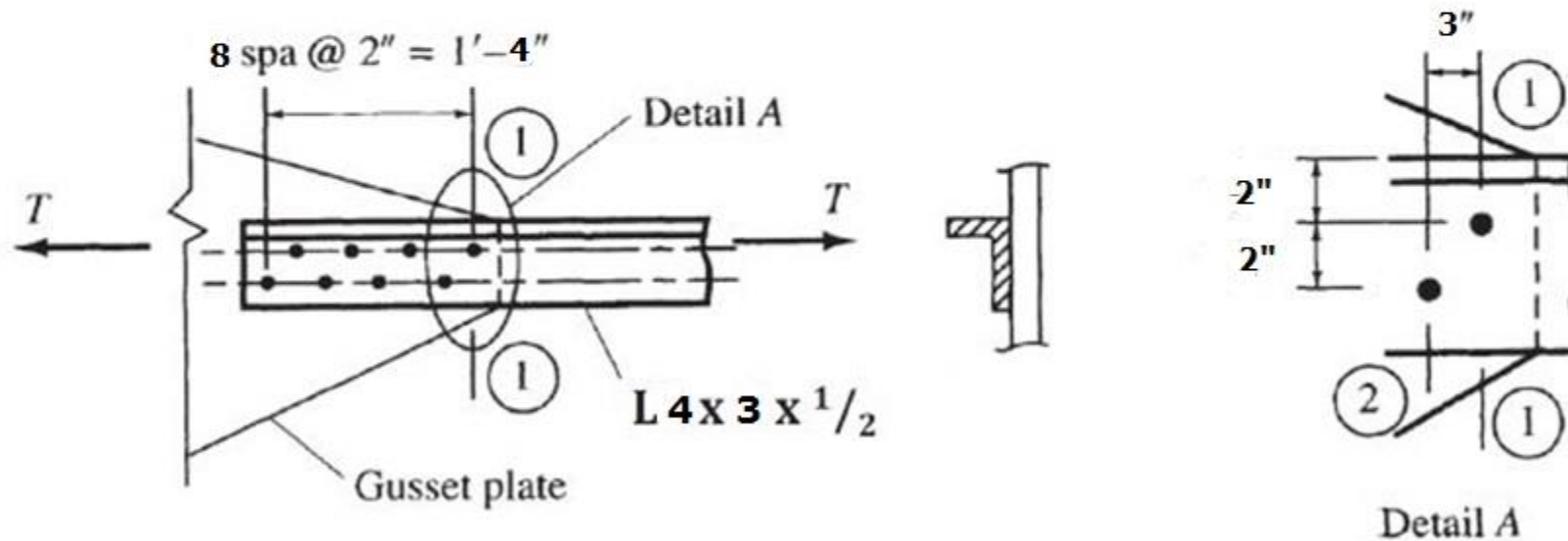
$$\Rightarrow D = 29.75 \text{ kips}$$

$$\therefore L = 3D = 89.25 \text{ kips}$$

# □ SUBMIT THE ASSIGNMENT IN MOODLE

## ASD Load Capacity

Determine the service load capacity in tension for an  $L 4 \times 3 \times 1/2$  A572 Grade 50 steel connected with  $5/8$ -in.-diam. bolts in standard holes as shown. Assume live load to be **two** times the dead load. Neglect block shear failure.





# SUBMIT THE ASSIGNMENT IN MOODLE

- USE ANGLE PROPERTY TABLE
- SOLVE THE MATH
- WRITE STUDENT ID
- TAKE PICTURE OF THE ASSIGNMENT
- UPLOAD IT IN THE MOODLE SITE.